

Sardar Patel College of Engineering

(A Government Aided Autonomous Institute)

Munshi Nagar, Andheri (West), Mumbai - 400058.

End Semester Examination

March - 2023

Max. Marks: 100 mrech civil with str. 6mgs

Im I 3/3/2

Duration: 3

Hours

Class.: M.Tech.

Semester: I

Program: Civil Engineering Course Code: PC-MST 101

Name of the Course: Structural Dynamics

Instructions:

• Answer any five questions.

• Answers to all sub questions should be grouped together.

• Figures to the right indicate full marks.

• Assume suitable data if necessary and state the same clearly.

Question No		Points	CO	BL	Module No.
Q1 (a)	What is dynamic load? Briefly explain the classification of dynamic loads with suitable example.	3	1	1,2	1
	For the rigid body system shown in figure: (a) Formulate the equation of motion	10	2	3	4
Q1(b)	(b) Determine the natural frequency and damping ratio Rigid bear max= m K K K L/2 L/2 M=800 kg m = 500 kg k = 1000 kg/m L/4	4/4	Rigid	plak G: m	
	K== 500 W/m C= 1 Kr-m/6 L=2m				
Q1(c)	A platform weighing 1500 N is supported on four columns. The columns are identical and clamped at both ends. It has been determined experimentally that a force of 250 KN horizontally applied to platform produces a displacement of 3.0 mm. Damping is 5%. Determine the following:	7	2	3	2

	(i) Stiffness of structure (ii) Damped frequency (ii) Damping coefficient (iii) Logarithmic decrement (iv)Number of cycles and time required for the amplitude of motion to be reduced from initial of 3.0 mm to 0.3 mm.				
	The frame shown in figure is subjected to a triangular pulse type load as shown in figure at girder level. Calculate the maximum horizontal displacement at girder level and maximum bending moment in column AB. The response spectra for this dynamic load are also shown in the figure.	8	2	3	2
Q2 (a)	Fo = 50 KJ	→ŧ			
Q2(b)	$E = R \times 10^{5} \text{ N/mm}$ $E = R \times 10^{5} \text{ N/mm}$ $T = 20,000 \text{ Cm}^{4}$ $Ref Last Parameter For Response special points of the second parameter for Transmissibility Ratio and briefly explain how vibration isolation can be achieved.$	ies de- ge 8	2	3	2
	explain now vibration isolation can be achieved.		 	 	
Q2(c)	A machine weighing 25 KN exerts harmonic force 4000 N amplitude, at 10 Hz at its supports. After installing the machine on a spring damper type isolator, the force exerted on the support is reduced to 400 N. Determine the spring stiffness k. Take damping ratio as 10%.	4	2	3	2
	A three storey single bay frame has storey height of 4 m. The columns on ground and first story are 250 mm wide X 600 mm deep while at 2 nd story the size a column is 250 mm x 500 mm & beams are very stiff. The mass on each and floor is	20	2	4	5
Q3	30 t. E = 20000 Mpa. Calculate natural frequencies & mode shapes				

	below is	subjected equency o	l to a har of 10 rad	ree vibration monic for sec. at the	ce with a 3 nd floo	mplitude r level an	100 Kn d 50 Kn	15	2	4	
Q4 (b)	amplitude maximun =5%	e with n displac	same fi ements	requency of each st	at 2 nd ory. Ta	floor. C ke dampi	Calculate ng ratio				
	Story No.	Mass No.	Mass (t)	ω rad/s ec		shapes	1.6	i			
		1	30	4.92	Φ_{i1} 0.336	Φ _{i2} 0.759	Φ _{i3}				
	$\frac{1}{2}$	2	30	13.45	-2.46	-0.804	1.0				
	3	3	30	$\frac{13.43}{18.7}$	1.58	-1.157	2.58				
	113	13	130	10.7	1.50	1.107	1 2.50	-			
<u> </u>				figure c		the fund	lamental	8	2	3	
Q 5(a)	nequenc	y using	Rayloig	מו היכיו מו היכיו		kg/m·	m = 200	Kel	L=4:		
Q 3(a)					تتكتن		m = 1 t-		- T	ل _ه .	
		$\frac{\lambda}{\alpha}$	L/2	L 1.	-12	my 6	E = 10	000 Ku	2		
		7		. /		1					
	A simply	v support	ed beam	of 8m spa	an. 300 i	nm wide	600 mm	12	2	4	
	`			pplied force							
	, -		-	displacem			_				
Q 5(b)				and shear		_					
				of mater							
	1	-	-	lowest co							
	Controus	tion itom		10 11050	IIII I O GEII	S modes	<u>-</u>		 		
	For the c	cantileve	· heam s	hown in fi	oure ca	culate th	e natural	10	2	4	
		ies and n			5 u. v, vu	wiwev WI	- 1144441441	1	~		
				-		form by	1 la				
Q6(a)		1	 نرر	= 100 kg/1	9	<u> </u>	' 12				
			4	·13.1.1.1.1.1. ·183		LL-18					
		γ		5 .1 1	t' -	2718	4.				
			= = 2X	mm/N 6		2 ^ 10 T		<u> </u>	 	 	
	1			06 (2) 3		11. 1		10	-	3	
	TC		rrea in	- , ,					2	3	
O(h)	If the b		OO KNI.	ነበብልክ የልለለ		mg, carec	nated the				
Q6(b)	constant	load of 1						1	I.	1 1	
Q6(b)	constant	load of 1		each mass.		·····		<u> </u>			
Q6(b)	constant maximum	load of I m repose	s under e	each mass.		ression fo	n natural	10	3	2	
	constant maximum	load of 1 m repose from firs	s under e	each mass.	the exp				3	2	
Q6(b) Q7(a)	constant maximum Starting frequence	from firsty and me	s under e st princip ode shap	each mass. oal, derive oe for a sir	the exp			1 -	3	2	
	constant maximum Starting frequence	load of 1 m repose from firs	s under e st princip ode shap	each mass. oal, derive oe for a sir	the exp			1 -	3	2	
	Starting frequence uniforml	from firs	s under e et princip ode shap uted mas	each mass. oal, derive oe for a sir	the expi	ported be	eam with		2,3	2	

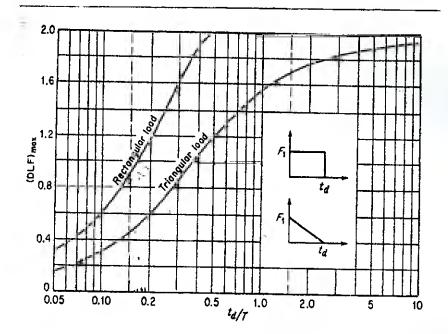


Figure for Q No.(a)



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End Sem March 2023

Program:

M.Tech-Structures

Course Code: PC -MST 102

Course Name: Advanced Structural Analysis

Duration: 3 Hrs

Maximum Points: 100

Semester: I

Q.No.	Questions	Points	СО	BL	Module No
	Analyze the following non-prismatic axially loaded structural system by stiffness method. Assume AE= 5000kN. Also support A and D slip 2mm and 1mm at right respectively.				
	A ZOKNÍM ZOKN A SM B AM E ZAE C				
Q1a	Find slope at free end D. Assume EI=10000kN/m ²	15	2	3	
	20kMm sokn sokn sokn sokn A 2m B 2m A 3m B				
)		05	1		
+	Analyze the following continuous beam using conventional stiffness method. Assume EI=80000kN-m2				
22	40kN/m 30kN A-4m A-3m b 2m A- A-C	20			



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	Analyze the following frame .Draw SFD and BMD.				
	Assume E= 30000 N/mm2. Size of element AB is	}			
	300mmX 300mm and BC is 300mmX 450mm.				
	who coke			İ	
	A South				
	Assume E= 30000 N/mm2. Size of element AB is 300mmX 300mm and BC is 300mmX 450mm. AOLW OKN SOLW SO		1		
	7				
	. (7)				
	1 25 KNM SICM				
	* 9				
Q3	777	20	1	3	
	A hook carries a load of 7.5kN and the load line is at a				
	distance of 20mm from the inner edge of section which is				
	trapezoidal. The load line also passes through the centre	1]
	of curvature of the hook. The dimensions of the central	j			
	horizontal trapezoidal section are inner width=50mm				
	,outer width=25mm.Depth=40mm.Calculate the	1			
	maximum and minimum stresses .Also plot the variation				
Q4	of stress across the section.	20	3	3	
	Analyze the following frame section. EI= 80000 kN-				
	m2,AE=5000kN				
	Support C slips to right by 5mm. 20 KN/W				
	C C				
	30kNm & Am		1	-	
	175 AM				
	30kNm 5/1				
	4	00	2	3	
<u>′Q5</u>	7.7	20		- 3	
	Calculate total strain energy stored in following frame.				
	120kNlm				}
	1. 1.201	1			<u> </u>
	1 Amount				ļ
	A) SM IM SIKN				
	A) Sm		1		
	2m			1	j
	Ava=300x500mm + 40kN				
	E= SOODOHIMM				•
	2:0.3 1 3m				
Q6	A'=A/1.2 20KN	20	1	3	<u> </u>
	The state of the s				

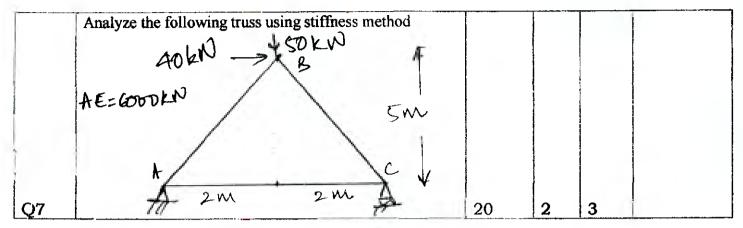


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End Sem March 2023





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End semester exam - March 2023

Program: M.Tech Civil Engineering - Structures Duration: 3 Hours

Maximum Points: 100

Course Name: Design of Prestressed Concrete

Semester: 1

Structures

Course Code: EC-MST 105

Notes:

- Attempt any 5 main questions. Draw neat sketches to illustrate your answers
- Assume suitable data if missing and state the same clearly.

• Use of IS 1343 is allowed

Q.No.	Questions	Points	СО	BL	Module
	a) Explain how pre-stressing affects deflection of beams. What are the factors affecting long term deflection?	05	1	2	01
	b) Explain: full pre-stressing, limited pre-stressing and partial pre-stressing.	05	l	2	01
1	c) A 6m long simply supported pre-stressed concrete beam has the cross section and pre-stressing profile as shown below. Determine the stresses in extreme fibres at mid span and supports if the beam carries a superimposed load of 15kN/m on full span.	10	1,3	3	02
_	40 ± 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6				
	Determine the ultimate moment of resistance for a prestressed box section as shown below. The $f_{ck} = 40 \text{ N/mm}^2$, $f_{pu} = 1800 \text{ N/mm}^2$. The total cross-sectional area of the tendons is 1000 mm2 with the centroid at a distance of 100 mm from the bottom fibre.				
2.a)	200 100 - 800 mm	10	3	3,5	03







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End semester exam - March 2023

	Dagion 41	o shoon	C					
2.b)	simply su 350mmx75 of 9.5kN/r cable that i fck 40Ml Effective p Characteris Use Fe415	apported beau 50mm and sp m(unfactored as having ecco Pa prestress in castic strength of	or of rectangul can 11m. It carries the line is prestress centricity of 275m able = 1200MPa of PT steel = 1600 for reinforcement.	OMPa	10	3	3,5	03
3.a)	with 2 cable is successive cables 1-2. Cable 1 Cable 2 Each cable initial tension co-efficient at transfer of Es = 210kN	Profile Parabolic Straight has a cross on of 1250M for wave effort for prestress = 1/mm², Ec = 3 ne % losses	Eccentricity at midspan 250mm (below N.A.) 450mm(below NA) section area of Pa. Co-efficient of fect = 0.0015/m. 28days. Anchor 0kN/mm².	Eccentricity at support Omm Omm Eccentricity at support Omm 450mm(below NA) 500mm and an for friction = 0.5; Age of concrete rage slip = 4mm.	12	2	3,5	02
3.b)	Explain the	effect of pre	_	ar resistance of a resses and Mohr	08	1,3	2	03
4.a)	pre-stressed	sections a	nd the IS code	red in design of provisions for post-tensioned	10	3,5	4	05
4.b)	Determine t for a simply i) Straight te ii) paraboli midspan	explain load-balancing concept. Determine the equivalent loading and the camber induced or a simply supported beam having a Straight tendon profile at an eccentricity e parabolic profile concentric at supports and e an idspan i) Inclined cable with a kink at midspan having					2	02
5.				beam consists of in-situ flange of	20	4	3	06



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	650mmX300mm. The stem is a post tensioned unit subjected to an effective prestressing force of 1000kN. The tendons are provided at 150mm from the soffit of stem. The beam has to support a live load of 12kN/m. Determine the resultant stress distribution in the beam if the beam is a) unpropped; b) propped Draw neat sketches to show the variations of stresses at each stage				
6.a)	Derive the expression for deflection due to prestress when the profile is parabolic having zero eccentricity at ends and "e" at mid span for a simply supported beam A simply supported pr-estressed beam of cross section 350mmX1200mm and span 15m has a straight profile of cable with eccentricity of 350mm below N.A. It carries a live load of 10kN/m. The area of cable is 500mm² and it is initially tensioned to 1250N/mm². % loss = 28% Calculate the: i) Instantaneous deflection due to dead load + prestressing force ii) Long term deflection if the creep coefficient is 1.6 Es=210kN/mm²; Ec =35kN/mm²	10	1	3	03
6.b)	The end block of a post-tensioned beam has three anchorages with 300 mm square bearing plates as shown in figure. An initial pre-stressing force of 700 kN is applied to each anchorage. Design the end zone reinforcement. 1200 mm 1200 mm 1500 1500 1500 1500 1500 1500	10	3	3,5	04







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		1		
100kN	1	150kN		
125mm		180mm		
	125mm	125mm	150kN 150kN 150mm	150kN 125nm 130mm







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End Semester Examinations- March 2023

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Program: M.Tech. (Structural Engineering) Lom I

Duration: 3 Hours

Maximum Points: 100

Semester: I

Course Code: EC-MST114

Course Name: Elective-II: Non Linear Analysis

Instructions:

• Attempt any FIVE questions out of SEVEN questions.

Answers to all sub questions should be grouped together.

• Figures to the right indicate full marks.

• Assume suitable data if necessary and state the same clearly.

Q.No	Questions	Points	со	BL	ΡI
Q1(a)	State and explain upper bound theorem used in plastic analysis.	(05)	1	1,2	1.3.1 2.1.3
Q1(b)	Explain St. Venant's torsion and warping torsion.	(05)	4	2	1.3.1
Q1(c)	Write a note lateral buckling of beams	(05)	4	1,2	1.3.1 2.1.3
Q1(d)	What is a beam column? Explain	(05)	3	2	1.3.1
Q2(a)	A steel beam of rectangular section 75 mm wide and 150 mm deep is simply supported over a span of 4 m. If the beam carries a uniformly distributed load of 30 kN / m length on the entire span, find the depth of the elastic core at the mid-span. The yield stress of steel 250 N/mm ² .	(06)	1	3,4	2.1.2
Q2(b)	Find the shape factor of a circular cross section of external D	(04)	1	3,4	2.1.2
Q2(c)	Find the shape factor of an unsymmetrical I section with following details: Top flange width = 400 mm & thickness = 40 mm Bottom flange width = 300 mm & thickness = 30 mm Depth of web = 300 mm and thickness of web = 30 mm	(10)	1	3,4	2.1.3 2.2.3



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Q3	For the frame shown in figure below, find the collapse load factor. Loads shown in the figure are working loads and the plastic moment capacity of each member in kN-m is also shown in the figure.	(20)	1	3,4	2.1.3 2.2.3
	40 kN 3m				
	125 125 80 kN 15 kN 3m				
	150 150 4m				
	min min				
Q4(a)	A continuous beam is subjected to working loads as shown in figure below. If $M_P = 80$ kN-m, calculate the (true) load factor for the beam.	(10)	1	1 3,4	2.1.3 2.2.3
	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$				
Q4(b)	Write a note on effect of axial force on plastic moment capacity of a flexural member.	(10)	2	1,2,	1.3.1 2.1.3
Q5(a)	A column of length L and pinned at both the ends is under the action of an axial compressive load P. The flexural rigidity of the member varies uniformly from EI at either end (support) to 1.5EI at the centre. Find the critical load by finite difference method.	(10)	3	3,4	2.2.3 2.4.1
Q5(b)	Use energy method A column of length L and pinned at both the ends is under the action of an axial compressive load P. Find the critical load by energy method if the flexural stiffness of the member varies according to $EI(x) = EI_0$ $0 \le x \le L/5$ $= 2EI_0$ $L/5 \le x \le 4L/5$ $= EI_0$ $4L/5 \le x \le L$	(10)	3	3,4	2.2.3 2.4.1



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Q6(a)	Determine the critical load for the frame shown in figure.	(14)	3	3,4	2.2.3 2.4.1
	P 3 m EI				
	3 m EI				
	mm.				
Q6(b)	Explain in case of a thin walled open cross section subjected to axial load, what are the possible modes of buckling if the section is (i) symmetrical about two perpendicular axes (ii) symmetrical about one axis (iii) unsymmetrical	(06)	4	1,2	1.3.1 2.1.3
Q7(a)	Derive the governing differential equation for the torsional buckling of column with symmetrical cross- section.	(14)	4	1,2,	1.3.1 2.1.3
Q7(b)	In case of lateral buckling of rectangular beam in pure bending, write the expression for critical stress and explain the terms involved in the expression.	(06)	4	1,2	1.3.1 2.1.3



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TERM END EXAMINATION MARCH 2023

Program: M. TECH (STRUCTURES) Sem I

1313/23 Dur

Duration: 3 HR

Course Code: EC-MST125

Maximum Points: 100

Course Name: Advanced solid Mechanics

Semester: I

Notes: Question no. 1 is compulsory. Solve any 2 questions out of remaining questions

Q.No.	Questions	Points	co	BL	PI
l a	The state of strain at a point is given by $\varepsilon x = 0.001$,				-
	$\varepsilon y = -0.003$, $\varepsilon z = 0$, $\gamma x y = 0$, $\gamma y z = 0.001$, $\gamma z x = -0.004$.				
	Determine the stress tensor at this point.	10	1	4	1.1.1
	Take $E = 210 \times 10^6 \text{ kN/m}^2$ Poisson's ratio = 0.28.		-		1.1.3
	Also find Lame's constant.				
1 b	State plane stress and plane strain. Discuss the plane				
	stress and plane strain for two dimensional problems with	10	1	4	1.1.1
	illustrations.				1.1.3
2	The stress field at a point with respect to X, Y, Z				
	coordinate system is given by the array in MPa as				
	$ \begin{bmatrix} 4 & 1 & 2 \\ 1 & 6 & 0 \\ 2 & 0 & 8 \end{bmatrix} $	20	1	4	1.1.1 1.1.3
	Show that by transformation of axis by 45° about the Z				
	axis in the anticlockwise direction, the stress invariants				
	remain unchanged.				
3	Show that for a simply supported beam, length 2L, depth				
	2a and unit width, loaded by a concentrated load W at the				
	centre, the stress function satisfying the loading condition				
	is \$= \frac{1}{6} xy^2 + C xey	20	2	4	1.1.1
	The positive direction of Y being upwards, and is at x				1.1.3
	0 at mid span. X axis is at centre of depth and towards				
	right.				





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4	For the beam shown determine the displacement field due	1	1		
	to bending only. Consider the cross section of beam to be rectangular and thin so that deflections are not functions of z. The stiffness of the beam is EIz and poisson's ratio v.	20	2	4	1.1.1 1.1.3
5 a	Determine the shear stress and angle of twist of the multi- cell given in figure subjected to torque 12 kNm. G = 90 GPa. Dimensions in mm	11	3	4	1.1.1 1.1.3
5 b	For the respective sections shown in figure below with shear stress restricted to 135 MPa and G as 70 GPa. Find the following (i) Torsional constant (ii) Torque (iii) Angle of twist	09	3	4	1.1.1 1.1.3





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ба	Determine the location of shear centre "e" for the cross section shown. All dimensions are in mm and thickness of walls is 4 mm.	12	3	4	1.1.1
6 b	For the given cable arrangement calculate deflection of point B in vertical and horizontal direction using complementary theorem $ \begin{array}{cccccccccccccccccccccccccccccccccc$	08	3	4	1.1.1 1.1.3
7	For plate loaded in tension by force per unit area σ , is having circular hole at centre. Outer diameter of the plate is very large compared to diameter of hole 2a. Use	20	3	4	1.1.1 1.1.3



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Bharatiya Vidya Bhavan's SARDAR PATEL COLLEGE OF ENGINEERING

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END SEMESTER EXAMINATION

Programme	MTech Structural Engineering	MTech Construction Management
Course Code	MC-PG01	MC-PG01

Mitech Str by Jan I 15/3/23

Subject: Research Methodology and IPR

Total Marks:100

Class: MTech, Sem:I,

Duration: 3 Hours 15th March 2022

- Question 1 is compulsory
- · Solve Any Four Questions from the remaining
- Answers to all sub questions must be grouped together
- Figures to the right indicate full marks

Assume suitable data wherever necessary

SN	Question statement	Points	Module	00
Q1A	Differentiate between Research Paper and Review paper. State the Guidelines to write the research article.	10	M2,M3	CO1, CO2
Q1B	State the difference between Copyright, Patent and Trademark. Draw the flow chart and explain the procedure to receive the patent.	10	M4, M5	CO2,
Q2A	 Explain the following with suitable examples Null Hypothesis and Alternate Hypothesis Type 1 error and Type 2 error Test Statistics Confidence Level and p value Limitations and advantages of Hypothesis Test Rejection Region Left Tail Test and Right Tail Test One Tail Test and Two Tail Test 	10	M1	CO1
Q2B	What are the research avenues in your branch of engineering? What may be research approach and methodology to deal with those? Explain how to conduct the literature survey in carrying out research? What is the difference between Invention, Discovery and Research?	10	M1, M2	CO1
Q3A	State the requirements of Good Research Problem. You are interested to submit the Research Proposal for Topic Approval. Prepare the list of contents of Research proposal and explain them	10	M1 M2,M4	CO3,

Q3B	Explain the following points with respect to Chi Square test.	10	M1,M2	CO1,
	Purpose of using Chi Sq Tests			CO3
	Chi Sq. Distribution	, I		
İ	Chi Sq.Table	ļ		
ŀ	Observed frequencies			}
	Estimated frequencies			
	Types of application of Chi Sq Test			
Q4A	A population is divided into four strata so that N1 = 9500, N2 = 5500	10	M3	CO2
	and N3 = 7000, N4 = 11500 Respective standard deviations are:			
Ī	s1=11, s2=15, s3=10, s4=7. Costs in rupees to collect the strata are C1=			
ĺ	10000, C2=5000, C3=7000, C4=11000. How should a sample of size n			
	= 92 be allocated to the four strata, if we want optimum allocation using			
	Cost Optimal Disproportionate sampling design?			
Q5A	Explain the following with suitable examples	10	M1,M5	CO3,
	A. Statistical Hypothesis Test Procedures and the Criminal Trial			CO1
	Analogy			
0.50	B. Patent Rights Geographical indications			~~1
Q5B	The following are the number of departmental stores in 15 cities: 35, 17,	10	M1,M2	CO1
	10, 32, 70, 28, 26, 19, 26, 66, 37, 44, 33, 29 and 28. If we want to select			~~~
	a sample of 25 stores, using cities as clusters and selecting within			CO2
	clusters proportional to size, how many stores from each city should be			
	chosen?(Use a starting point of 5).			
Q6A	What are the prerequisites of data collection? Explore the factors	10	M1	CO1,
	affecting success of interview process. Differentiate the Structured			
	interview, Semi-structured interview and unstructured interview process			CO2
	with suitable examples. Refer the following points			
	 Knowledge of Interviewer Time availability for both parties 			
	Cost		1	
	Bias			
	Freedom of expression			
	Area of application			
	Quality of output			
]	Noise factors		1	
	Knowledge of Interviewee			
	Efforts needed for conducting interview			
Q6B	Researcher conducted experimental investigations on concrete cubes, to	10	M1,	CO2
	study the influence of fly ash, GGBS and glass waste powder (GWP)		M5	
	individually, on the compressive strength of concrete. The cubes were		1	
	casted for M30 grade of concrete and by random sampling method,			
	tested after 28 days curing. For cubes in Group I, 30% fly ash was			
	added, for Group II, 30% GGBS was added and in Group III, 30% GWP was added. The 28 days compressive strengths of cubes in N/mm ²		0.00	
<u> </u>	GWT was added. The 20 days compressive strengths of cubes in hymm		1	

	are given below. 6 different groups is Group I – 34, 33, Group II – 32, 29, Group III – 34, 32	s same or 29, 35, 28, 35, 29, 3 2, 29, 35,							
Q7A	know association between type of location from which the candidate applied and success in interview. The response by type of location is as follows.						10	M1, M5	CO2
		Rural area	Urban area	Total					
	Successful in Interview	28	44	72		!			
	Not successful in interview	20	32	52					
	Total	48	76	124					
	At alpha =0.05, 0 of location of can								
Q7B	Manufacturer w						10	M1	CO1,
	determinations ar				_				CO2
		hermal conductivity of a certain kind of plate is 0.34 units, as has be							
	claimed. The me								
}	in similar studi								
	determinations is necessary.	given t	by $\sigma = 0$.	Ul. Ass	sume any suitab	le data if			

Standard Normal Probabilities

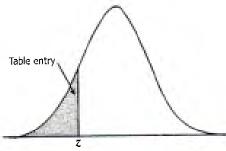
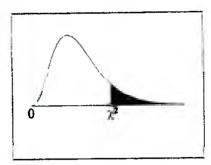


Table entry for z is the area under the standard normal curve to the left of z.

z	.00	.01	.02	.03	.04	.05	.06	.07	.08_	.09
-3,4	.0003	.0003	.0003	.0003	,0003	.0003	.0003	.0003	.0003	.0002
-3.3	.0005	.0005	.0005	.0004	.0004	.0004	.0004	.0004	.0004	.0003
-3.2	-0007	.0007	.0006	.0006	.0006	.0006	.0006	.0005	.0005	.0005
-3.1	.0010	.0009	.0009	.0009	.0008	.0008	.0008	.0008	,0007	.0007
-3.0	.0013	.0013	.0013	.0012	.0012	.0011	.0011	.0011	.0010	.0010
-2.9	.0019	.0018	.0018	.0017	.0016	.0016	.0015	.0015	.0014	.0014
-2.8	,0026	,0025	.0024	.0023	.0023	.0022	.0 021	.0021	.0020	.0019
-2.7	.0035	.0034	.0033	.0032	.0031	.0030	.0029	.0028	.0027	.0026
-2.6	.0047	.0045	.0044	.0043	.0041	.0040	.0039	.0038	.0037	.0036
-2.5	.0062	.0060	.0059	.0057	.0055	.0054	.0052	.0051	:0049	.0048
-2.4	.0082	.0080	.0078	.0075	.0073	.0071	.0069	.0068	.0066	.0064
-2.3	.0107	0104	.0102	.0099	.0096	.0094	.0091	.0089	.0087	.0084
-2.2	.0139	.0136	.0132	.0129	.0125	.0122	.0119	.0116	.0113	.0110
-2.1	.0179	.0174	.0170	.0166	.0162	.0158	.0154	,0150	.0146	.0143
-2.0	.0228	.0222	.0217	.0212	.0207	.0202	.0197	.0192	.0188	.0183
-1.9	,0287	.0281	.0274	.0268	.0262	.02 5 6	.0 250	.0244	.0239	.0233
-1.8	.0359	.0351	.0344	.0336	.0329	.0322	.0314	.0307	.0301	.0294
-1.7	.0446	.0436	.0427	.0418	.0409	.0401	.0392	.0384	.0375	.0367
-1.6	.0548	.0537	.0526	.0516	.0505	.0495	.0485	.0475	.0465	.0455
-1.5	.0668	.0655	.0643	.0630	.0618	.0606	0594	.0582	.0571	.0559
-1.4	.0808	.0793	.0778	.0764	.0749	.0735	.0721	.0708	.0694	.0681
-1.3	.0968	.0951	.0934	.0918	.0901	.0885	.0869	.0853	.0838	.0823
-1.2	1151	1131	1112	.1093	.1075	1056	.1038	.1020	1003	.0985
-1.1	.1357	.1335	.1314	1292	.1271	1251	.1230	1210	1190	1170
-1.0	₋₁₅₈₇	-1562	.1539	.1515	.1492	.1469	1446	1423	1401	.1379
-0.9	1841	1814	.1788	.1762	1736	.1711	1685	.1660	.1635	.1611
-0.8	.2119	.2090	.2061	-2033	2005	1977	1949	1922	1894	1867
-0.7	.2420	.2389	.2358	.2327	.2296	.2266	.2236	.2206	.2177	.2148
-0.6	.2743	.2709	.2676	.2643	.2611	.2578	.2546	.2514	2483	.2451
-0.5	.3085	.3050	.3015	2981	,2946	.2912	.2877	.2843		.2776
-0.4	,3446	.3409	.3372	.3336	.3300	.3264	.3228	.3192	.3156	.3121
-0.3	.3821	.3783	.3745	.3707	.3669	.3632	.3594	.3557	.3520	.3483
-0.2	.4207	.4168	.4129	.4090	.4052	.4013	.3974	.3936	.3897	.3859
-0.1	.4602	.4562	,4522	.4483	,4443	.4404	4364	.4325	4286	.4247
-0.0	.5000	.49 60	.4920	.4880	.4840	.4801	.4761	.4721	,4681	.4641

Chi-Square Distribution Table



The shaded area is equal to α for $\chi^2=\chi^2_{\alpha}$.

1 0.000 0.001 0.004 0.016 2.706 3.841 5.024 6.635 2 0.010 0.020 0.051 0.103 0.211 4.605 5.991 7.378 9.210 3 0.072 0.115 0.216 0.352 0.584 6.251 7.815 9.348 11.345 4 0.207 0.297 0.484 0.711 1.064 7.779 9.488 11.143 13.277 5 0.412 0.554 0.831 1.145 1.610 9.236 11.070 12.833 15.086 6 0.676 0.872 1.237 1.635 2.204 10.645 12.592 14.449 16.812 7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.475 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.90 9 1.735 2.088 2.70	T .		_								,
2 0.010 0.020 0.051 0.103 0.211 4.605 5.991 7.378 9.210 3 0.072 0.115 0.216 0.352 0.584 6.251 7.815 9.348 11.345 4 0.207 0.297 0.484 0.711 1.064 7.779 9.488 11.143 13.277 5 0.412 0.554 0.831 1.145 1.610 9.236 11.070 12.833 15.086 6 0.676 0.872 1.237 1.635 2.204 10.645 12.592 14.449 16.812 7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.475 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.90 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 <td< th=""><th>X.005</th><th>$\chi^2_{.010}$</th><th>$\chi^2_{.025}$</th><th>$\chi^{2}_{.050}$</th><th>χ2,100</th><th>$\chi^2_{.900}$</th><th>$\chi^2_{.950}$</th><th>$\chi^2_{.975}$</th><th>$\chi^2_{.990}$</th><th>$\chi^{2}_{.995}$</th><th>df</th></td<>	X.005	$\chi^2_{.010}$	$\chi^2_{.025}$	$\chi^{2}_{.050}$	χ2,100	$\chi^2_{.900}$	$\chi^2_{.950}$	$\chi^2_{.975}$	$\chi^2_{.990}$	$\chi^{2}_{.995}$	df
2 0.010 0.020 0.051 0.103 0.211 4.605 5.991 7.378 9.210 3 0.072 0.115 0.216 0.352 0.584 6.251 7.815 9.348 11.345 4 0.207 0.297 0.484 0.711 1.064 7.779 9.488 11.143 13.277 5 0.412 0.554 0.831 1.145 1.610 9.236 11.070 12.833 15.086 6 0.676 0.872 1.237 1.635 2.204 10.645 12.592 14.449 16.812 7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.475 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.090 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 <t< td=""><td>7.879</td><td>6:635</td><td>5.024</td><td>3.841</td><td>2.706</td><td>0.016</td><td>0.004</td><td>0.001</td><td>0.000</td><td>0.000</td><td>1</td></t<>	7.879	6:635	5.024	3.841	2.706	0.016	0.004	0.001	0.000	0.000	1
3 0.072 0.115 0.216 0.352 0.584 6.251 7.815 9.348 11.345 4 0.207 0.297 0.484 0.711 1.064 7.779 9.488 11.143 13.277 5 0.412 0.554 0.831 1.145 1.610 9.236 11.070 12.833 15.086 6 0.676 0.872 1.237 1.635 2.204 10.645 12.592 14.449 16.812 7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.475 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.090 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 2.558 3.247 3.940 4.865 15.987 18.307 20.483 23.209 11 2.603	10.597	9.210	7.378	5.991	4.605						
4 0.207 0.297 0.484 0.711 1.064 7.779 9.488 11.143 13.277 5 0.412 0.554 0.831 1.145 1.610 9.236 11.070 12.833 15.086 6 0.676 0.872 1.237 1.635 2.204 10.645 12.592 14.449 16.812 7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.476 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.090 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 2.558 3.247 3.940 4.865 15.987 18.307 20.483 23.209 11 2.603 3.053 3.816 4.575 5.578 17.275 19.675 21.920 24.725 12 3.074	12.838	11.345	9.348	7.815							
5 0.412 0.554 0.831 1.145 1.610 9.236 11.070 12.833 15.086 6 0.676 0.872 1.237 1.635 2.204 10.645 12.592 14.449 16.812 7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.475 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.090 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 2.558 3.247 3.940 4.865 15.987 18.307 20.483 23.209 11 2.603 3.053 3.816 4.575 5.578 17.275 19.675 21.920 24.725 12 3.074 3.571 4.404 5.226 6.304 18.549 21.026 23.337 26.217 13 3.565 <td>14.860</td> <td>13.277</td> <td>11.143</td> <td>9.488</td> <td>7.779</td> <td>1.064</td> <td></td> <td>0.484</td> <td></td> <td></td> <td></td>	14.860	13.277	11.143	9.488	7.779	1.064		0.484			
6 0.676 0.872 1.237 1.635 2.204 10.645 12.592 14.449 16.812 7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.475 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.090 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 2.558 3.247 3.940 4.865 15.987 18.307 20.483 23.209 11 2.603 3.053 3.816 4.575 5.578 17.275 19.675 21.920 24.725 12 3.074 3.571 4.404 5.226 6.304 18.549 21.026 23.337 26.217 13 3.565 4.107 5.009 5.892 7.042 19.812 22.362 24.736 27.688 14 4.075 </td <td>16.750</td> <td>15.086</td> <td>12.833</td> <td>11.070</td> <td>9.236</td> <td>1.610</td> <td>1.145</td> <td>0.831</td> <td></td> <td></td> <td>1</td>	16.750	15.086	12.833	11.070	9.236	1.610	1.145	0.831			1
7 0.989 1.239 1.690 2.167 2.833 12.017 14.067 16.013 18.475 8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.090 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 2.558 3.247 3.940 4.865 15.987 18.307 20.483 23.209 11 2.603 3.053 3.816 4.575 5.578 17.275 19.675 21.920 24.725 12 3.074 3.571 4.404 5.226 6.304 18.549 21.026 23.337 26.217 13 3.565 4.107 5.009 5.892 7.042 19.812 22.362 24.736 27.688 14 4.075 4.660 5.629 6.571 7.790 21.064 23.685 26.119 29.141 15 4.601<	18.548	16.812	14.449	12.592	10.645	2.204	1.635				
8 1.344 1.646 2.180 2.733 3.490 13.362 15.507 17.535 20.090 9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 2.558 3.247 3.940 4.865 15.987 18.307 20.483 23.209 11 2.603 3.053 3.816 4.575 5.578 17.275 19.675 21.920 24.725 12 3.074 3.571 4.404 5.226 6.304 18.549 21.026 23.337 26.217 13 3.565 4.107 5.009 5.892 7.042 19.812 22.362 24.736 27.688 14 4.075 4.660 5.629 6.571 7.790 21.064 23.685 26.119 29.141 15 4.601 5.229 6.262 7.261 8.547 22.307 24.996 27.488 30.578 16 5.142	20.278	18.475	16.013	14.067	12.017	2.833					1
9 1.735 2.088 2.700 3.325 4.168 14.684 16.919 19.023 21.666 10 2.156 2.558 3.247 3.940 4.865 15.987 18.307 20.483 23.209 11 2.603 3.053 3.816 4.575 5.578 17.275 19.675 21.920 24.725 12 3.074 3.571 4.404 5.226 6.304 18.549 21.026 23.337 26.217 13 3.565 4.107 5.009 5.892 7.042 19.812 22.362 24.736 27.688 14 4.075 4.660 5.629 6.571 7.790 21.064 23.685 26.119 29.141 15 4.601 5.229 6.262 7.261 8.547 22.307 24.996 27.488 30.578 16 5.142 5.812 6.908 7.962 9.312 23.542 26.296 28.845 32.000 17 5.69	21.955	20.090	17.535	15.507	13.362						1
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	23.589	21.666	19.023	16.919	14.684	4.168					
12 3.074 3.571 4.404 5.226 6.304 18.549 21.026 23.337 26.217 13 3.565 4.107 5.009 5.892 7.042 19.812 22.362 24.736 27.688 14 4.075 4.660 5.629 6.571 7.790 21.064 23.685 26.119 29.141 15 4.601 5.229 6.262 7.261 8.547 22.307 24.996 27.488 30.578 16 5.142 5.812 6.908 7.962 9.312 23.542 26.296 28.845 32.000 17 5.697 6.408 7.564 8.672 10.085 24.769 27.587 30.191 33.409 18 6.265 7.015 8.231 9.390 10.865 25.989 28.869 31.526 34.805 19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 <td< td=""><td>25.188</td><td>23.209</td><td>20.483</td><td>18.307</td><td>15.987</td><td>4.865</td><td>3.940</td><td>3.247</td><td></td><td></td><td>10</td></td<>	25.188	23.209	20.483	18.307	15.987	4.865	3.940	3.247			10
13 3.565 4.107 5.009 5.892 7.042 19.812 22.362 24.736 27.688 14 4.075 4.660 5.629 6.571 7.790 21.064 23.685 26.119 29.141 15 4.601 5.229 6.262 7.261 8.547 22.307 24.996 27.488 30.578 16 5.142 5.812 6.908 7.962 9.312 23.542 26.296 28.845 32.000 17 5.697 6.408 7.564 8.672 10.085 24.769 27.587 30.191 33.409 18 6.265 7.015 8.231 9.390 10.865 25.989 28.869 31.526 34.805 19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 <	26.757	24.725	21.920	19.675	17.275	5.578	4.575	3.816	3.053	2.603	11
14 4.075 4.660 5.629 6.571 7.790 21.064 23.685 26.119 29.141 15 4.601 5.229 6.262 7.261 8.547 22.307 24.996 27.488 30.578 16 5.142 5.812 6.908 7.962 9.312 23.542 26.296 28.845 32.000 17 5.697 6.408 7.564 8.672 10.085 24.769 27.587 30.191 33.409 18 6.265 7.015 8.231 9.390 10.865 25.989 28.869 31.526 34.805 19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22	28.300	26.217	23.337	21.026	18.549	6.304	5.226	4.404	3.571		
14 4.075 4.660 5.629 6.571 7.790 21.064 23.685 26.119 29.141 15 4.601 5.229 6.262 7.261 8.547 22.307 24.996 27.488 30.578 16 5.142 5.812 6.908 7.962 9.312 23.542 26.296 28.845 32.000 17 5.697 6.408 7.564 8.672 10.085 24.769 27.587 30.191 33.409 18 6.265 7.015 8.231 9.390 10.865 25.989 28.869 31.526 34.805 19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22	29.819	27.688	24.736	22.362	19.812	7.042	5.892	5.009	4.107	3.565	13
16 5.142 5.812 6.908 7.962 9.312 23.542 26.296 28.845 32.000 17 5.697 6.408 7.564 8.672 10.085 24.769 27.587 30.191 33.409 18 6.265 7.015 8.231 9.390 10.865 25.989 28.869 31.526 34.805 19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22 8.643 9.542 10.982 12.338 14.041 30.813 33.924 36.781 40.289 23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24	31.319	29.141	26.119	23.685	21:064	7.790		5.629	4.660		14
17 5.697 6.408 7.564 8.672 10.085 24.769 27.587 30.191 33.409 18 6.265 7.015 8.231 9.390 10.865 25.989 28.869 31.526 34.805 19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22 8.643 9.542 10.982 12.338 14.041 30.813 33.924 36.781 40.289 23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25<	32.801	30.578	27.488	24.996	22.307	8.547	7.261	6.262		4.601	15
17 5.697 6.408 7.564 8.672 10.085 24.769 27.587 30.191 33.409 18 6.265 7.015 8.231 9.390 10.865 25.989 28.869 31.526 34.805 19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22 8.643 9.542 10.982 12.338 14.041 30.813 33.924 36.781 40.289 23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25<	34.267	32.000	28.845	26.296	23.542	9.312	7.962	6.908	5. 812	5.142	16
19 6.844 7.633 8.907 10.117 11.651 27.204 30.144 32.852 36.191 20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22 8.643 9.542 10.982 12.338 14.041 30.813 33.924 36.781 40.289 23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25 10.520 11.524 13.120 14.611 16.473 34.382 37.652 40.646 44.314 26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642	35.718	33.409	30.191	27.587	24.769	10.085	8.672	7.564	6.408	5.697	17
20 7.434 8.260 9.591 10.851 12.443 28.412 31.410 34.170 37.566 21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22 8.643 9.542 10.982 12.338 14.041 30.813 33.924 36.781 40.289 23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25 10.520 11.524 13.120 14.611 16.473 34.382 37.652 40.646 44.314 26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642 27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963	37.156	34.805	31.526	28.869	25.989	10.865	9.390	8.231	7.015	6.265	18
21 8.034 8.897 10.283 11.591 13.240 29.615 32.671 35.479 38.932 22 8.643 9.542 10.982 12.338 14.041 30.813 33.924 36.781 40.289 23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25 10.520 11.524 13.120 14.611 16.473 34.382 37.652 40.646 44.314 26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642 27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963 28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	38.582	36.191	32.852	30.144	27.204	11.651	10.117	8.907	7.633	6.844	19
22 8.643 9.542 10.982 12.338 14.041 30.813 33.924 36.781 40.289 23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25 10.520 11.524 13.120 14.611 16.473 34.382 37.652 40.646 44.314 26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642 27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963 28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	39.997	37.566	34.170	31.410	28.412	12.443	10.851	9.591	8.260	7.434	20
23 9.260 10.196 11.689 13.091 14.848 32.007 35.172 38.076 41.638 24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25 10.520 11.524 13.120 14.611 16.473 34.382 37.652 40.646 44.314 26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642 27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963 28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	41.401	38.932	35.479	32.671	29.615	13.240	11.591	10.283			21
24 9.886 10.856 12.401 13.848 15.659 33.196 36.415 39.364 42.980 25 10.520 11.524 13.120 14.611 16.473 34.382 37.652 40.646 44.314 26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642 27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963 28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	42.796	40.289	36.781	33.924	30.813	14.041	12.338	10.982			
25 10.520 11.524 13.120 14.611 16.473 34.382 37.652 40.646 44.314 26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642 27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963 28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	44.181	41.638	38.076	35.172	32.007	14.848	13.091	14.689	10.196	9.260	2 3
26 11.160 12.198 13.844 15.379 17.292 35.563 38.885 41.923 45.642 27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963 28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	45.559	42.980	39.364	36.415	33.196	15.659					
27 11.808 12.879 14.573 16.151 18.114 36.741 40.113 43.195 46.963 28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	46.928	44.314	40.646	37.652	34.382	16.473	14.611				
28 12.461 13.565 15.308 16.928 18.939 37.916 41.337 44.461 48.278	48.290	45.642	41.923	38.885	35.563						
	49.645	46.963	43.195	40.113	36.741		16.151				
	50.993.	48.278	44.461	41.337	37.916	18.939					
29 13.121 14.256 16.047 17.708 19.768 39.087 42.557 45.722 49.588	52.336	49.588	45.722	42.557		19.768	,		,		4
30 13.787 14.953 16.791 18.493 20.599 40.256 43.773 46.979 50.892	53.672	50.892	46.979	43.773	40.256	20.599	18.493	16.791	14.953		
40 20.707 22.164 24.433 26.509 29.051 51.805 55.758 59.342 63.691	66.766	63.691	59.342	55.758	51.805						
50 27.991 29.707 32.357 34.764 37.689 63.167 67.505 71.420 76.154	79.490	76.154	71.420	67.505							
60 35.534 37.485 40.482 43.188 46.459 74.397 79.082 83.298 88.379	91.952	88.379	83.298	79.082							
70 43.275 45.442 48.758 51.739 55.329 85.527 90.531 95.023 100.425											L
80 51.172 53.540 57.153 60.391 64.278 96.578 101.879 106.629 112.329	116.321	112.329	106.629								
90 59.196 61.754 65.647 69.126 73.291 107.565 113.145 118.136 124.116	128.299	124.116	118.136	113.145	107.565						
100 67.328 70.065 74.222 77.929 82.358 118.498 124.342 129.561 135.807			129.561	124.342	118.498	82.358	77.929	74.222	70.065	67.328	100

Standard Normal Probabilities

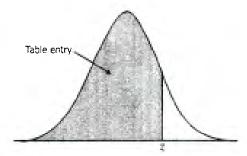


Table entry for z is the area under the standard normal curve to the left of z.

	.00	.01	.02	.03	.04	.05	.06	.07	.08	.09
0.0	.5000	-5040	.5080	.5120	.5160	.5199	.5239	.5279	.5319	.5359
0.1	.5398	.5438	.5478	5517	5557	,5596	.5636	.5675	.5714	.5753
0.2	5793	-5832	.5871	5910	.5948	.5 9 87	.6026	.6064	.6103	.6141
0.3	.6179	.6217	.6255	.6293	.6331	.6368	.6406	.6443	,6480	.651%
0.4	.6554	.6591	.6628	.6664	.6700	.6736	.6772	.6808	.6844	.6879
0.5	.6915	6950	.6985	.7019	.7054	.7088	.7123	.7157	.7190	.7224
0.6	.7257	.7291	.7324	.7357	.7389	.7422	.7454	.7486	.7517	.7549
0.7	.7580	.7611	.7642	.7673	.7704	.7734	.7764	.7794	.7823	.7852
0.8	.7881	.7910	.7939	.7967	.7 9 95	.8023	.8051	.80 78	.8106	.8133
0.9	.8159	.8186	,8212	.8238	.8264	.8289	.8315	.8340	.8365	.8389
1.0	.8413	.8438	.8461	.8485	.8508	.8531	.8554	.8577	.859 9	.8621
1.1	.8643	.8665	.8686	.8708	.8729	.8749	8770	.8790	.8810	.8830
1.2	.8849	.8869	.8888	.8907	.8925	.8944	.8962	.8980	.8997	.9015
1.3	.9032	,9049	.9066	.9082	.9099	.9115	.9131	.9147	.9162	.9177
1.4	.9192	.9207	.9222	.9236	.9251	.9265	.9279	.9292	.9306	.9319
1,5	.9332	.9345	.9357	.9370	.9382	.9394	.9406	.9418	.9429	.9441
1.6	.9452	.9463	.9474	.9484	.9495	.9505	,9515	.9525	.9 53 5	.9545
1.7	.9554	.9564	.9573	.9582	.9591	9599	.9608	.9616	.9625	.9633
1.8	.9641	.9649	.9656	.9664	.9671	.9678	.9686	.9693	.9699	.970 6
1.9	.9713	.9719	.9726	.9732	.9738	.9744	.9750	.9756	.9761	.9767
2.0	.9772	.9778	.9783	.9788	.9793	.9798	.9803	.9808	.9812	.9817
2.1	.9821	.9826	.9830	.9834	.9838	.9842	.9846	.9850	.9854	.9857
2,2	.9861	.9864	98 68	.9871	.9875	.9878	.9881	.9884	.9 887	.9890
2.3	.9893	.9896	.9898	.9901	.9904	.9906	.9909	.9911	.9913	10040
2,4	.9918	.9920	.9922	.9925	.9927	9929	.99 31	.9932	.9934	.9936
2.5	.9938	.9940	.9941	.9943	.9945	.9946	9948	.9949	.9951	.9952
2.6	.9953	.9955	.9956	.9957	.9959	.9960	.9961	.9962	.9963	.9964
2.7	.9965	.9966	.9967	.9968	.9969	.9970	.9971	.9972	.9973	.9974
2.8	.9974	.9975	.9976	.9 9 77	.9977	.9978	. 9 979	.997 9	.9980	.9981
2.9	.9981	.9982	.9982	.9983	.9984	.9984	.9985	.9985	.9986	.9986
3.0	.9987	.9987	.9987	.9988	.9988	.9989	.9989	.9989	.9990	.9990
3.1	.9990	.9991	.9 9 91	.9991	- .99 92	.9992	. 9 992	.9992	.9993	.9993
3.2	.99 93	9993	. 99 94	.9994	.99 9 4	.9994	.9994	.9995	.9995	.9995
3.3	.9995	,9995	.9995	.9996	.9996	.9996	.99 9 6	.9996	.9996	.9997
3.4	.9997	,9997	. 9 997	.99 9 7	.9997	.9997	.9 997	. 99 97	.9997	.9998



SARDAR PATEL COLLEGE OF ENGINEERING

(Government Aided Autonomous Institute) Munshi Nagar, Andheri (W) Mumbai – 400058



END SEMESTER EXAMINATION MARCH 2023

Program: F. Y. M. Tech Will with Str Engl Lewis Duration: 3 Hours

Course Code: AU-PG 01 Maximum Points: 100

Course Name: Project Planning and Management Semester: I

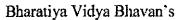
Notes:

1. Answer any five questions.

2. All questions carry 20 points.

Q.No.	Questions	Points	СО	BL	Module No.
	1.1 Why is it important to issue a Civil, Structural,				
	Architectural Design Basis for the project? List ten of				
	the most important design requirements that should be				
1	contained in the Design Basis.	10	1	1	2
	1.2 List out at least ten steps, in sequence, for the				
	preparation of Civil Structural, Architectural Tender				
	specification.	10	2	2	2
	2.1 Explain the three fundamental components in a			ļ	
	computer model for structural analysis. Explain the				
	three stages in the process of computer analysis				
2	highlighting Engineer's and computer's roles.	10	2	3	3
	2.2 List out at least ten steps, in sequence, in the		1		
	designing of a complex plant steel structure.		ļ		
		10	2	2	4
	3.1 Write a detailed note on reinforcement Bar				
	Bending Schedules including contents, cutting length and users.				
3		10	2	1	5
	3.2 Explain Building Information Modelling (BIM)				
	along with its use in different stages of a construction				
	project and its advantages.	10	2	3	4
	4.1 Explain the key concepts for Project Resource				
4	Management and Project Risk Management.	10	1	2	2
-	4.2 Explain any five top Emerging Trends which are		1		
	impacting the Construction Industry today.	10	3	5	5
		1 10	1,5		Page 1 of 2

Page 1 of 2







(Government Aided Autonomous Institute) Munshi Nagar, Andheri (W) Mumbai – 400058

END SEMESTER EXAMINATION MARCH 2023

Q.No.	Questions	Points	со	BL	Module No.
	5.1 Explain the change in approach towards Project				
	Management in the seventh edition of the PMBOK				<u>}</u>
5	Guide briefly defining project management principles and performance domains; list any three of each.	10	3	2	1
	5.2 List ten guidelines to be considered while				
	developing a plot plan/layout for a process plant.	10	2	3	2
	6.1 Write a brief note on i)Project and ii)Project				
6	Management	10	2	1	2
	6.2 Explain Quality and Grade. Explain Quality				
	Assurance and Quality Control; which is preferred				
	and why?	10	4	4	5
	7.1 When a project commences, what are the early				
	activities carried out by CSA discipline? On what				
	activities does the CSA engineer spend a major				
7	portion of time?	10	4	2	1
	7.2 Write a note on Method statement in construction.				
	What are the objectives of Constructability Reviews?	10	2	3	2