## Bharatiya Vidya Bhavan's <br> Sardar Patel College of Engineering

(A Government Aided Autonomous Institute)
Munshi Nagar, Andheri (West), Mumbai - 400058.
End Semester Examination
March - 2023
Max. Marks: 100 mi tech civic with str. Lings - Lem i 3/3/23
Hours
Class: M.Tech.
Narrie of the Course: Structural Dynamics

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In structions:

- Answer any five questions.
- Answers to all sub questions should be grouped together.
- Figures to the right indicate full marks.
- Assume suitable data if necessary and state the same clearly.


|  | (i) Stiffness of structure (ii) Damped frequency (ii) Damping coefficient (iii) Logarithmic decrement (iv)Number of cycles and time required for the amplitude of motion to be reduced from initial of 3.0 mm to 0.3 mm . |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Q2 (a) | The frame shown in figure is subjected to a triangular pulse type load as shown in figure at girder level. Calculate the maximum horizontal displacement at girder level and maximum bending moment in column AB . The response spectra for this dynamic load are also shown in the figure. |  | 2 | 3 | 2 |
| Q2(b) | Derive the expression for Transmissibility Ratio and briefly explain how vibration isolation can be achieved. | 8 | 2 | 3 | 2 |
| Q2(c) | A machine weighing 25 KN exerts harmonic force 4000 N amplitude, at 10 Hz at its supports. After installing the machine on a spring damper type isolator, the force exerted on the support is reduced to 400 N . Determine the spring stiffness k. Take damping ratio as $10 \%$. | 4 | 2 | 3 | 2 |
| Q3 | A three storey single bay frame has storey height of 4 m . The columns on ground and first story are 250 mm wide $\mathbf{X} 600$ mm deep while at $2^{\text {nd }}$ story the size a column is 250 mm x 500 mm \& beams are very stiff. The mass on each and floor is $\mathbf{3 0} \mathbf{t} . \mathbf{E}=\mathbf{2 0 0 0 0} \mathbf{M p a}$. Calculate natural frequencies \& mode shapes | 20 | 2 | 4 | 5 |
| Q4(a) | State and prove orthogonality principle. Also state the significance of orthogonality principle in dynamic analysis | 5 | 2 | 3 | 5 |
|  |  |  |  |  |  |




Figure for QND.(a)
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Program: M.Tech-Structuxes
Course Code: PC -MST 102
Course Name: Advanced Structural Analysis

Duration: 3 Hrs
Maximum Points: 100
Semester: I


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| Q3 | Analyze the following frame .Draw SFD and BMD. Assume $\mathrm{E}=30000 \mathrm{~N} / \mathrm{mm} 2$. Size of element AB is 300 mmX 300 mm and BC is 300 mmX 450 mm . | 20 | 1 | 3 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Q4 | A hook carries a load of 7.5 kN and the load line is at a distance of 20 nam from the inner edge of section which is trapezoidal. The load line also passes through the centre of curvature of the hook. The dimensions of the central horizontal trapezoidal section are inner width $=50 \mathrm{~mm}$ ,outer width $=25 \mathrm{~mm}$. Depth $=40 \mathrm{~mm}$. Calculate the maximum and minimum stresses. Also plot the variation of stress across the section. | 20 | 3 | 3 |  |
| 45 | Analyze the following frame section. $\mathrm{EI}=80000 \mathrm{kN}$ $\mathrm{m} 2 . \mathrm{AE}=5000 \mathrm{kN}$ <br> Support C slips to right by 5 mm . | 20 | 2 | 3 |  |
| Q6 | Calculate total strain energy stored in following frame. | 20 | 1 | 3 | - |

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| Q7 | Analyze the following truss using stiffness method | 20 | 2 | 3 |
| :---: | :---: | :---: | :---: | :---: |

End semester exam - March 2023

Program: M.Tech Civil Engineering - Structures
Course Code: EC-MST 105
Course Name: Design of Prestressed Concrete Structures

## Notes:

- Attempt any 5 main questions. Draw neat sketches to illustrate your answers
- Assume suitable data if missing and state the same clearly.
- Use of IS 1343 is allowed

| Q.No. | Questions | Points | CO | BL | Module |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | a) Explain how pre-stressing affects deflection of beams. What are the factors affecting long term deflection? <br> b) Explain : full pre-stressing, limited pre-stressing and partial pre-stressing. <br> c) A 6 m long simply supported pre-stressed concrete beam has the cross section and pre-stressing profile as shown below. Determine the stresses in extreme fibres at mid span and supports if the beam carries a superimposed load of $15 \mathrm{kN} / \mathrm{m}$ on full span. | 05 <br> 05 <br> 10 | I $1,3$ | 2 <br> 2 <br> 3 | 01 <br> 01 <br> 02 |
| 2.a) | Determine the ultimate moment of resistance for a prestressed box section as shown below. The $f_{c x}=40 \mathrm{~N} / \mathrm{mm}^{2}$, $f_{p u}=1800 \mathrm{~N} / \mathrm{mm}^{2}$. The total cross-sectional area of the tendons is 1000 mm 2 with the centroid at a distance of 100 mm from the bottom fibre. | 10 | 3 | 3,5 | 03 |

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| 2.b) | Design the shear reinforcement at quarter span for a simply supported beam of rectangular cross section 350 mmx 750 mm and span 11 m . It carries a live load UDL of $9.5 \mathrm{kN} / \mathrm{m}$ (unfactored). It is prestressed by a straight cable that is having eccentricity of 275 mm fck $=40 \mathrm{MPa}$ <br> Effective prestress in cable $=1200 \mathrm{MPa}$ <br> Characteristic strength of PT steel $=1600 \mathrm{MPa}$ <br> Use Fe 415 grade steel for reinforcement. <br> Assume that the section is cracked. |  |  |  | 10 | 3 | 3,5 | 03 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3.a) | a) A sim with 2 ca is success cables 1-2 $\square$ <br> Cable I <br> Cable 2 <br> Each cab initial ten co-efficien at transfe $\mathrm{Es}=210 \mathrm{k}$ Calculate and ancho | y supported es having a ely tension <br> Profile <br> Parabolic <br> Straight <br> has a cros <br> of 1250 M <br> for wave <br> f prestress <br> $/ \mathrm{mm}^{2}$, Ec <br> \% losses <br> ge slip | post tensioned b ross section of 3 from a single e <br> Eccentricity at midspan <br> 250 mm <br> (below N.A.) <br> 450mm(below NA) <br> section area of <br> Pa. Co-efficient ect $=0.0015 / \mathrm{m}$. 28days. Ancho $0 \mathrm{kN} / \mathrm{mm}^{2}$. <br> due to elastic sh | am of span 20 m 0 mmX 1400 mm ad in the order of <br> Eccentricity at support 0 mm <br> 450mm(below NA) <br> 500 mm and an for friction $=0.5$; Age of concrete age slip $=4 \mathrm{~mm}$. <br> rtening, friction | 12 | 2 | 3,5 | 02 |
| 3.b) | Explain the effect of pre-stressing on shear resistance of a beam using the concept of principal stresses and Mohr circle |  |  |  | 08 | 1,3 | 2 | 03 |
| 4.a) | Explain the various stages to be considered in design of pre-stressed sections and the IS code provisions for limiting stresses for pre-tensioned and post-tensioned members. |  |  |  | 10 | 3,5 | 4 | 05 |
| 4.b) | Explain load-balancing concept. <br> Determine the equivalent loading and the camber induced for a simply supported beam having <br> i) Straight tendon profile at an eccentricity e <br> ii) parabolic profile concentric at supports and e at midspan <br> iii) Inclined cable with a kink at midspan having eccentricity e |  |  |  | 10 | 1 | 2 | 02 |
| 5. | A 15 m span simply supported composite beam consists of 350 mmX 600 mm precast stem and a cast-in-situ flange of |  |  |  | 20 | 4 | 3 | 06 |


|  | 650 mmX 300 mm . The stem is a post tensioned unit subjected to an effective prestressing force of 1000 kN . The tendons are provided at 150 mm from the soffit of stem. The beam has to support a live load of $12 \mathrm{kN} / \mathrm{m}$. Determine the resultant stress distribution in the beam if the beam is a) unpropped; b) propped <br> Draw neat sketches to show the variations of stresses at each stage |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 6.a) | Derive the expression for deflection due to prestress when the profile is parabolic having zero eccentricity at ends and "e" at mid span for a simply supported beam <br> A simply supported pr-estressed beam of cross section $350 \mathrm{~mm} \times 1200 \mathrm{~mm}$ and span 15 m has a straight profile of cable with eccentricity of 350 mm below N.A. It carries a live load of $10 \mathrm{kN} / \mathrm{m}$. The area of cable is $500 \mathrm{~mm}^{2}$ and it is initially tensioned to $1250 \mathrm{~N} / \mathrm{mm}^{2} . \%$ loss $=28 \%$ Calculate the : <br> i) Instantaneous deflection due to dead load + prestressing force <br> ii) Long term deflection if the creep coefficient is 1.6 <br> $E s=210 \mathrm{kN} / \mathrm{mm}^{2} ; \mathrm{Ec}=35 \mathrm{kN} / \mathrm{mm}^{2}$ | 10 | 1 | 3 | 03 |
| 6.b) | The end block of a post-tensioned beam has three anchorages with 300 mm square bearing plates as shown in figure. An initial pre-stressing force of 700 kN is applied to each anchorage. Design the end zone reinforcement. | 10 | 3 | 3,5 | 04 |

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End semester exam - March 2023


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End Semester Examinations- March 2023
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Program: M.Tech. (Structural Engineering) Lem I
Course Code: EC-MST114
Course Name: Elective-II: Non Linear Analysis

Duration: 3 Hours
Maximum Points: 100
Semester: I

## Instructions:

- Attempt any FIVE questions out of SEVEN questions.
- Answers to all sub questions should be grouped together.
- Figures to the right indicate full marks.
- Assume suitable data if necessary and state the same clearly.

| Q.No | Questions | Points | co | BL | PI |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Q1(a) | State and explain upper bound theorem used in plastic analysis. | (05) | 1 | 1,2 | $\begin{aligned} & 1.3 .1 \\ & 2.1 .3 \end{aligned}$ |
| Q1(b) | Explain St. Venant's torsion and warping torsion. | (05) | 4 | 2 | 1.3.1 |
| Q1(c) | Write a note lateral buckling of beams | (05) | 4 | 1,2 | $\begin{array}{\|l\|} \hline 1.3 .1 \\ 2.1 .3 \\ \hline \end{array}$ |
| Q1(d) | What is a beam column? Explain | (05) | 3 | 2 | 1.3.1 |
| Q2(a) | A steel beam of rectangular section 75 mm wide and 150 mm deep is simply supported over a span of 4 m . If the beam carries a uniformly distributed load of $30 \mathrm{kN} / \mathrm{m}$ length on the entire span, find the depth of the elastic core at the mid-span. The yield stress of steel $250 \mathrm{~N} / \mathrm{mm}^{2}$. | (06) | 1 | 3,4 | 2.1.2 |
| Q2(b) | Find the shape factor of a circular cross section of external D | (04) | 1 | 3,4 | 2.1 .2 |
| Q2(c) | Find the shape factor of an unsymmetrical I section with following details: <br> Top flange width $=400 \mathrm{~mm}$ \& thickness $=40 \mathrm{~mm}$ <br> Bottom flange width $=300 \mathrm{~mm}$ \& thickness $=30 \mathrm{~mm}$ <br> Depth of web $=300 \mathrm{~mm}$ and thickness of web $=30 \mathrm{~mm}$ | (10) | 1 | 3,4 | $\begin{array}{\|l\|} \hline 2.1 .3 \\ 2.2 .3 \end{array}$ |

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| Q3 | For the frame shown in figure below, find the collapse load factor. Loads shown in the figure are working loads and the plastic moment capacity of each member in $\mathrm{kN}-\mathrm{m}$ is also shown in the figure. | (20) | 1 | 3,4 | $\begin{aligned} & \hline 2.1 .3 \\ & 2.2 .3 \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |
| Q4(a) | A continuous beam is subjected to working loads as shown in figure below. If $\mathrm{M}_{\mathrm{P}}=80 \mathrm{kN}-\mathrm{m}$, calculate the (true) load factor for the beam. | (10) | 1 | 3,4 | $\begin{array}{\|l\|l} \hline 2.1 .3 \\ 2.2 .3 \end{array}$ |
|  |  |  |  |  |  |
| Q4(b) | Write a note on effect of axial force on plastic moment capacity of a flexural member. | (10) | 2 | $\begin{aligned} & 1,2, \\ & 3, \end{aligned}$ | $\begin{array}{\|l\|} \hline 1.3 .1 \\ 2.1 .3 \end{array}$ |
| Q5(a) | A column of length $L$ and pinned at both the ends is under the action of an axial compressive load $P$. The flexural rigidity of the member varies uniformly from EI at either end (support) to 1.5 EI at the centre. Find the critical load by finite difference method. | (10) | 3 | 3,4 | $\begin{array}{\|l} 2.2 .3 \\ 2.4 .1 \end{array}$ |
| Q5(b) | Use energy method A column of length $L$ and pinned at both the ends is under the action of an axial compressive load P. Find the critical load by energy method if the flexural stiffness of the member varies according to $\begin{aligned} \mathrm{EI}(\mathrm{x}) & =\mathrm{EI}_{0} & & 0 \leq \mathrm{x} \leq \mathrm{L} / 5 \\ & =2 \mathrm{EI}_{0} & & \mathrm{~L} / 5 \leq \mathrm{x} \leq 4 \mathrm{~L} / 5 \\ & =\mathrm{EI}_{0} & & 4 \mathrm{~L} / 5 \leq \mathrm{x} \leq \mathrm{L} \end{aligned}$ | (10) | 3 | 3,4 | $\begin{array}{\|l} 2.2 .3 \\ 2.4 .1 \end{array}$ |

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| Q6(a) | Determine the critical load for the frame shown in figure. | (14) | 3 |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
|  |  |  |  |  |  |

Course Code: EC-MST125
Course Name: Advanced solid Mechanics

Maximum Points: 100
Semester: I

Notes: Question no. 1 is compulsory. Solve any 2 questions out of remaining questions

| Q.No. | Questions | Points | CO | BL | PI |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 a | The state of strain at a point is given by $\varepsilon x=0.001$, $\varepsilon y=-0.003, \varepsilon z=0, \gamma x y=0, \gamma y z=0.001, \gamma z x=-0.004$. <br> Determine the stress tensor at this point. <br> Take $\mathrm{E}=210 \times 10^{6} \mathrm{kN} / \mathrm{m}^{2}$ Poisson's ratio $=0.28$. <br> Also find Lame's constant. | 10 | 1 | 4 | $\begin{aligned} & 1.1 .1 \\ & 1.1 .3 \end{aligned}$ |
| 1 b | State plane stress and plane strain. Discuss the plane stress and plane strain for two dimensional problems with illustrations. | 10 | 1 | 4 | 1.1.1 |
| 2 | The stress field at a point with respect to $X, Y, Z$ coordinate system is given by the array in MPa as $\left[\begin{array}{lll} 4 & 1 & 2 \\ 1 & 6 & 0 \\ 2 & 0 & 8 \end{array}\right]$ <br> Show that by transformation of axis by $45^{\circ}$ about the $Z$ axis in the anticlockwise direction, the stress invariants remain unchanged. | 20 | 1 | 4 | $\begin{aligned} & 1.1 .1 \\ & 1.1 .3 \end{aligned}$ |
| 3 | Show that for a simply supported beam, length 2L, depth 2a and unit width, loaded by a concentrated load W at the centre, the stress function satisfying the loading condition is $\phi=\frac{b}{6} x y^{2}+C x y$ <br> The positive direction of Y being upwards, and is at $\mathrm{x}=$ 0 at mid span. X axis is at centre of depth and towards right. | 20 | 2 |  | $\begin{aligned} & 1.1 .1 \\ & 1.1 .3 \end{aligned}$ |

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# Bharatiya Vida Bhavan's SARDAR PATEL COLLEGE OF ENGINEERING 

(An Autonomous Institution Affiliated to University of Mumbai)
END SEMESTER EXAMINATION

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| Programme | MTech <br> Structural Engineering | MTech <br> Construction <br> Management |
| :--- | :--- | :--- |
| Course Code | MC-PG01 | MC-PGU1 |

Duration : 3 Hours
$15{ }^{\text {th }}$ March 2022

- Question 1 is compulsory
- Solve Any Four Questions from the remaining
- Answers to all sub questions must be grouped together
- Figures to the right indicate full marks
- Assume suitable data wherever necessary


| Q3B | Explain the following points with respect to Chi Square test. <br> - Purpose of using Chi Sq Tests <br> - Chi Sq. Distribution <br> - Chi Sq.Table <br> - Observed frequencies <br> - Estimated frequencies <br> - Types of application of Chi Sq Test | 10 | M1,M2 | $\begin{aligned} & \mathrm{CO1}, \\ & \mathrm{CO} \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: |
| Q4A | A population is divided into four strata so that $\mathrm{N} 1=9500, \mathrm{~N} 2=5500$ and $\mathrm{N} 3=7000, \mathrm{~N} 4=11500$ Respective standard deviations are: $\mathrm{sl}=11, \mathrm{~s} 2=15, \mathrm{~s} 3=10, \mathrm{~s} 4=7$. Costs in rupees to collect the strata are $\mathrm{Cl}=$ $10000, \mathrm{C} 2=5000, \mathrm{C} 3=7000, \mathrm{C} 4=11000$. How should a sample of size n $=92$ be allocated to the four strata, if we want optimum allocation using Cost Optimal Disproportionate sampling design? | 10 | M3 | CO2 |
| Q5A | Explain the following with suitable examples <br> A. Statistical Hypothesis Test Procedures and the Criminal Trial Analogy <br> B. Patent Rights Geographical indications | 10 | M1,M5 | $\begin{aligned} & \mathrm{CO3} \\ & \mathrm{CO} 1 \end{aligned}$ |
| Q5B | The following are the number of departmental stores in 15 cities: 35,17 , $10,32,70,28,26,19,26,66,37,44,33,29$ and 28 . If we want to select a sample of 25 stores, using cities as clusters and selecting within clusters proportional to size, how many stores from each city should be chosen?(Use a starting point of 5). | 10 | M1,M2 | $\begin{aligned} & \mathrm{CO1} \\ & \mathrm{CO} 2 \end{aligned}$ |
| Q6A | What are the prerequisites of data collection? Explore the factors affecting success of interview process. Differentiate the Structured interview, Semi-structured interview and unstructured interview process with suitable examples. Refer the following points <br> - Knowledge of Interviewer <br> - Time availability for both parties <br> - Cost <br> - Bias <br> - Freedom of expression <br> - Area of application <br> - Quality of output <br> - Noise factors <br> - Knowledge of Interviewee <br> - Efforts needed for conducting interview | 10 | M1 | $\begin{aligned} & \mathrm{CO1} \\ & \mathrm{CO} 2 \end{aligned}$ |
| Q6B | Researcher conducted experimental investigations on concrete cubes, to study the influence of fly ash, GGBS and glass waste powder (GWP) individually, on the compressive strength of concrete. The cubes were casted for M30 grade of concrete and by random sampling method, tested after 28 days curing. For cubes in Group I, 30\% fly ash was added, for Group II; $30 \%$ GGBS was added and in Group III, 30\% GWP was added. The 28 days compressive strengths of cubes in $\mathrm{N} / \mathrm{mm}^{2}$ | 10 | $\begin{aligned} & \text { M1, } \\ & \text { M5 } \end{aligned}$ | C02 |

$\left.\begin{array}{|l|l|l|l|l|}\hline & \begin{array}{l}\text { are given below. Check whether the mean compressive strength of the 3 } \\ \text { different groups is same or not. } \\ \text { Group I }-34,33,29,35,28,31 \\ \text { Group II }-32,29,35,29,35\end{array} \\ \text { Group III }-34,32,29,35,33,29,28\end{array}\right)$

## Standard Normal Probabilities



Table entry for $z$ is the area under the standard normal curve to the left of $z$.

| $z$ | . 00 | . 01 | . 02 | . 03 | . 04 | . 05 | . 06 | . 07 | . 08 | . 09 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| -3.4 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0002 |
| -3.3 | . 0005 | . 0005 | .0605 | . 0004 | . 0004 | . 00004 | . 0004 | . 0004 | . 0004 | . 0003 |
| -3.2 | . 0007 | . 0007 | . 0006 | . 0006 | . 0006 | . 0006 | . 0006 | . 0005 | . 0005 | . 0005 |
| $-3.1$ | . 0010 | . 0009 | 00009 | .0009 | .0008 | . 0008 | . 0008 | . 00008 | . 0007 | . 0007 |
| -3.0 | . 0013 | . 0013 | . 0013 | . 0012 | . 0012 | . 0011 | . 0011 | . 0011 | . 0010 | . 0010 |
| $-2.9$ | . 0019 | . 0018 | 0018 | . 0017 | . 0016 | . 0016 | . 0015 | . 0015 | . 0014 | . 0014 |
| $-2.8$ | 0026 | . 0025 | . 0024 | . 0023 | . 0023 | . 0022 | . 0021 | . 0021 | . 0020 | . 0019 |
| -2.7 | . 0035 | . 0034 | . 0033 | . 0032 | . 0031 | . 0030 | . 0029 | . 0028 | . 0027 | . 0026 |
| -2.6 | . 0047 | . 0045 | . 0044 | . 0043 | . 0041 | . 0040 | . 0039 | . 0038 | . 0037 | . 0036 |
| -2.5 | . 0062 | . 0060 | 00059 | . 0057 | . 0055 | . 0054 | . 0052 | . 0051 | ,0049 | . 0048 |
| -2.4 | . 0082 | . 0080 | . 0078 | . 0075 | . 0073 | . 0071 | . 0069 | . 0068 | . 0066 | . 0064 |
| $-2.3$ | . 0107 | 0104 | . 0102 | . 0099 | . 0096 | . 0094 | . 0091 | . 0089 | . 0087 | .0084: |
| $-2.2$ | . 0139 | . 0136 | . 0132 | . 0129 | . 0125 | . 0122 | . 0119 | . 0116 | . 0113 | . 0110 |
| $-2.1$ | . 0179 | . 0174 | . 0170 | . 0166 | . 0162 | . 0158 | . 0154 | . 0150 | . 0146 | . 0143 |
| $-2.0$ | . 0228 | . 0222 | . 0217 | . 0212 | . 0207 | . 0202 | . 0197 | . 0192 | . 0188 | . 0183 |
| -1.9 | . 0287 | . 0281 | . 0274 | . 0268 | . 0262 | . 0256 | . 0250 | . 0244 | . 0239 | . 0233 |
| -1.8 | . 0359 | . 0351 | . 0344 | . 0336 | . 0329 | . 0322 | . 0314 | . 0307 | . 0301 | . 0294 |
| -1.7 | . 0446 | . 0436 | . 0427 | . 0418 | . 0409 | . 040.1 | . 0392 | . 0384 | . 0375 | 0367 |
| $-1.6$ | . 0548 | . 0537 | . 0526 | . 0516 | . 0505 | . 0495 | . 0485 | . 0475 | . 0465 | . 0455 |
| -1.5 | . 0668 | . 0655 | . 0643 | . 0630 | .0618 | . 0606 | . 0594 | 0582 | .,0571 | 0559 |
| -1.4 | . 0808 | . 0793 | . 0778 | . 0764 | . 0749 | . 0735 | . 0721 | . 0708 | . 0694 | . 0681 |
| -1.3 | . 0968 | . 0951 | . 0934 | . 0918 | . 0901 | . 0885 | . 0869 | . 0853 | . 0838 | . 0823 |
| -1.2 | . 1151 | .1131 | . 1112 | . 1093 | . 1075 | 1056 | 1038 | . 1020 | . 1003 | . 0985 |
| -1.1 | . 1357 | . 1335 | . 1314 | . 1292 | .1271 | 1251 | .1230 | 1210 | . 1190 | . 1170 |
| -1.0 | . 1587 | . 1562 | . 1539 | . 1515 | . 1492 | . 1469 | . 1446 | .1423 | . 1401 | . 1379 |
| -0.9 | 1841 | . 1814 | . 1788 | .1762 | . 1736 | . 1711 | . 1685 | . 1660 | . 1635 | 1611 |
| -0.8 | . 2119 | . 2090 | . 2061 | . 2033 | . 2005 | 1977 | . 1949 | . 1922 | . 1894 | . 1867 |
| -0.7 | . 2420 | . 2389 | . 2358 | . 2327 | . 2296 | . 2266 | . 2236 | . 2206 | . 2177 | . 2148 |
| -0.6 | . 2743 | . 2709 | . 2676 | . 2643 | 2611 | . 2578 | . 2546 | . 2514 | .2483 | . 2451 |
| -0.5 | . 3085 | . 3050 | . 3015 | 2981 | . 29446 | . 2912 | . 2877 | . 2843 | . 2810 | .2776 |
| -0.4 | . 3446 | . 3409 | . 3372 | . 3336 | . 3300 | . 3264 | . 3228 | . 3192 | . 3156 | . 3121 |
| -0.3 | . 3821 | . 3783 | . 3745 | . 3707 | . 3669 | . 3632 | . 3594 | . 3557 | .3520 | 3483 |
| -0.2 | . 4207 | . 4168 | . 4129 | . 4090 | . 4052 | . 4013 | . 3974 | . 3936 | . 3897 | . 3859 |
| -0.1 | . 4602 | . 4562 | . 4522 | . 4483 | . 4443 | . 4404 | . 4364 | . 4325 | . 4286 | . 4247 |
| -0.0 | . 5000 | . 4960 | .4920 | . 4880 | . 4840 | . 4801 | . 4761 | . 4721 | . 4681 | . 4641 |

## Chi-Square Distribution Table



The shaded area is equal to $\alpha$ for $\chi^{2}=\chi_{\alpha}^{2}$.

| df | $\chi^{2} .995$ | $\chi^{2} .990$ | $\chi^{2}{ }^{2}{ }^{\text {a }}$ | $\chi_{\text {. }}^{250}$ | $\chi^{2}{ }^{2} 000$ | $\chi^{2} 100$ | $\chi^{2} .050$ | $\chi^{2} .025$ | $\chi^{2} .10$ | $\chi^{2} .005$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 0.000 | 0.000 | 0.001 | 0.004 | 0.016 | 2.706 | 3.841 | 5.024 | 6:635 | 7.879 |
| 2 | 0.010 | 0.020 | 0.051 | 0.103 | 0.211 | 4.605 | 5.991 | 7.378 | 9.210 | 10.597 |
| 3 | 0.072 | 0.115 | 0.216 | 0.352 | 0.584 | 6.251 | 7.815 | 9.348 | 11.345 | 12.838 |
| 4 | 0.207 | 0.297 | 0.484 | 0.711 | 1.064 | 7.779 | 9.488 | 11.143 | 13.277 | 14.860 |
| 5 | 0.412 | 0.554 | 0.831 | 1.145 | 1.610 | 9.236 | 11.070 | 12.833 | 15.086 | 16.750 |
| 6 | 0.676 | 0.872 | 1.237 | 1.635 | 2.204 | 10.645 | 12.592 | 14.449 | 16.812 | 18.548 |
| 7 | 0.989 | 1.239 | 1.690 | 2.167 | 2.833 | 12.017 | 14.067 | 16.013 | 18.475 | 20.278 |
| 8 | 1.344 | 1.646 | 2.180 | 2.733 | 3.490 | 13.362 | 15.507 | 17.535 | 20.090 | 21.955 |
| 9 | 1.735 | 2.088 | 2.700 | 3.325 | 4.168 | 14.684 | 16.919 | 19.023 | 21.666 | 23.589 |
| 10 | 2.156 | 2.558 | 3.247 | 3.940 | 4.865 | 1.5.987 | 18.307 | 20.483 | 23.209 | 25.188 |
| 11 | 2.603 | 3.053 | 3.816 | 4.575 | 5.578 | 17.275 | 19.675 | 21.920 | 24.725 | 26.757 |
| 12 | 3.074 | 3.571 | 4.404 | 5.226 | 6.304 | 18.549 | 21.026 | 23.337 | 26.217 | 28.300 |
| 13 | 3.565 | 4.107 | 5.009 | 5.892 | 7.042 | 19.812 | 22.362 | 24.736 | 27.688 | 29.819 |
| 14 | 4.075 | 4.660 | 5.629 | 6.571 | 7.790 | 21.064 | 23.685 | 26.119 | 29.141 | 31.319 |
| 15 | 4.601 | 5.229 | 6.262 | 7.261 | 8.547 | 22.307 | 24.996 | 27.488 | 30.578 | 32.801 |
| 16 | 5.142 | 5.812 | 6.908 | 7.962 | 9.312 | 23.542 | 26.296 | 28.845 | 32.000 | 34.267 |
| 17 | 5.697 | 6.408 | 7.564 | 8.672 | 10.085 | 24.769 | 27.587 | 30.191 | 33.409 | 35.718 |
| 18 | 6.265 | 7.015 | 8.231 | . 390 | 10.865 | 25.989 | 28.869 | 31.526 | 34.805 | 37.156 |
| 19 | 6.844 | 7.633 | 8.907 | 10.117 | 11.651 | 27.204 | 30.144 | 32.852 | 36.191 | 38.582 |
| 20 | 7.434 | 8.260 | 9.591 | 10.851 | 12.443 | 28.412 | 31.410 | 34.170 | 37.566 | 39.997 |
| 21 | 8.034 | 8.897 | 10.283 | 11.591 | 13.240 | 29.615 | 32.671 | 35.479 | 38.932 | 41.401 |
| 22 | 8.643 | 9.542 | 10.982 | 12.338 | 14.041 | 30.813 | 33.924 | 36.781 | 40.289 | 42.796 |
| 23 | 9.260 | 10.196 | 11.689 | 13.091 | 14.848 | 32.007 | 35.172 | 38.076 | 41.638 | 44.181 |
| 24 | 9.886 | 10.856 | 12.401 | 13.848 | 15.659 | 33.196 | 36.415 | 39.364 | 42.980 | 45.559 |
| 25 | 10.520 | 11.524 | 13.120 | 14.611 | 16.473 | 34.382 | 37.652 | 40.646 | 44.314 | 46.928 |
| 26 | 11.160 | 12.198 | 13.844 | 15.379 | 17.292 | 35.563 | 38.885 | 41.923 | 45.642 | 48.290 |
| 27 | 11.808 | 12.879 | 14.573 | 16.151 | 18.114 | 36.741 | 40.113 | 43.195 | 46.963 | 49.645 |
| 28 | 12.461 | 13.565 | 15.308 | 16.928 | 18.939 | 37.916 | 41.337 | 44.461 | 48.278 | 50.993 |
| 29 | 13.121 | 14.256 | 16.047 | 17.708 | 19.768 | 39.087 | 42.557 | 45.722 | 49.588 | 52.336 |
| 30 | 13.787 | 14.953 | 16.791 | 18.493 | 20.599 | 40.256 | 43.773 | 46.979 | 50.892 | 53.672 |
| 40 | 20.707 | 22.164 | 24.433 | 26.509 | 29.051 | 51.805 | 55.758 | 59.342 | 63.691 | 66.766 |
| 50 | 27.991 | 29.707 | 32.357 | 34.764 | 37.689 | 63.167 | 67.505 | 71.420 | 76.154 | 79.490 |
| 60 | 35.534 | 37.485 | 40.482 | 43.188 | 46.459 | 74.397 | 79.082 | 83.298 | 88.379 | 91.952 |
| 70 | 43.275 | 45.442 | 48.758 | 51.739 | 55.329 | 85.527 | 90.531 | 95.023 | 100.425 | 104.215 |
| 80 | 51.172 | 53.540 | 57.153 | 60.391 | 64.278 | 96.578 | 101.879 | 106.629 | 112.329 | 116.321 |
| 90 | 59.196 | 61.754 | 65.647 | 69.126 | 73.291 | 107.565 | 113.145 | 118.136 | 124.116 | 128.299 |
| 100 | 67.328 | 70.065 | 74.222 | 77.929 | 82.358 | 118.498 | 124.342 | 129.561 | 135.807 | 140.169 |

## Standard Normal Probabilities



Table entry for $z$ is the area under the standard normal curve to the left of $z$.

| $\underline{7}$ | . 00 | . 01 | . 02 | . 03 | . 04 | . 05 | . 06 | . 07 | . 08 | . 09 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.0 | . 5000 | . 5040 | . 5080 | . 5120 | . 5160 | . 5199 | . 5239 | . 5279 | . 5319 | . 5359 |
| 0.1 | . 5398 | . 5438 | . 5478 | . 5517 | . 5557 | . 5596 | . 5636 | . 5675 | . 5714 | . 5753 |
| 0.2 | . 5793 | . 5832 | . 5871 | . 5910 | . 5948 | . 5987 | . 6026 | . 6064 | . 6103 | . 6141 |
| 0.3 | . 6179 | . 6217 | . 6255 | . 6293 | . 6331 | . 6368 | . 6406 | . 6443 | .6480 | ,6594\% |
| 0.4 | . 6554 | . 6591 | . 6628 | . 6664 | . 6700 | . 6736 | . 6772 | . 6808 | . 6844 | . 6879 |
| 0.5 | . 6915 | . 6950 | . 6985 | . 7019 | . 7054 | . 7088 | . 7123 | . 7157 | . 7190 | . 7224 |
| 0.6 | . 7257 | . 7291 | . 7324 | . 7357 | . 7389 | . 7422 | . 7454 | . 7486 | . 7517 | . 7549 |
| 0.7 | . 7580 | 7611 | . 7642 | . 7673 | . 7704 | . 7734 | . 7764 | . 7794 | . 7823 | .7852 |
| 0.8 | . 7881 | . 7910 | . 7939 | . 7967 | . 7995 | . 8023 | . 8051 | . 8078 | . 8106 | . 8133 |
| 0.9 | . 8159 | . 8186 | . 8212 | . 8238 | . 8264 | . 8289 | . 8315 | . 8340 | . 8365 | . 8389 |
| 1.0 | . 8413 | . 8438 | . 8461 | . 8485 | . 8508 | . 8531 | . 8554 | . 8577 | . 8599 | . 8621 |
| 1.1 | . 8643 | . 8665 | . 8688 | . 8708 | . 8729 | . 8749 | - 8770 | . 8790 | . 8810 | 8820 |
| 1.2 | . 8849 | . 8869 | . 8888 | . 8907 | . 8925 | . 8944 | . 8962 | . 8980 | . 8997 | . 9015 |
| 1.3 | . 9032 | . 9049 | . 9066 | . 9082 | . 9099 | . 9115 | . 9131 | . 9147 | . 9162 | . 9177 |
| 1.4 | . 9192 | . 9207 | . 9222 | . 9236 | . 9251 | . 9265 | . 9279 | . 9292 | . 9306 | . 9319 |
| 1.5 | . 9332 | . 9345 | . 9357 | . 9370 | . 9382 | . 9394 | . 9406 | . 9418 | . 9429 | . 9441 |
| 1.6 | . 9452 | . 9463 | . 9474 | . 9484 | . 9495 | . 9505 | . 9515 | . 9525 | . 9535 | . 9545 |
| 1.7 | . 9554 | . 9564 | . 9573 | . 9582 | . 9591 | . 9599 | . 9608 | . 9616 | . 9625 | . 9633 |
| 1.8 | . 9641 | . 9649 | . 9656 | . 9664 | . 9671 | . 9678 | . 9686 | . 9693 | . 9699 | . 9706 |
| 1.9 | . 9713 | . 9719 | . 9726 | . 9732 | . 9738 | . 9744 | . 9750 | . 9756 | . 9761 | . 9767 |
| 2.0 | . 9772 | . 9778 | . 9783 | . 9788 | . 9793 | . 9798 | . 9803 | . 9808 | . 9812 | . 9817 |
| 2.1 | . 9821 | . 9826 | 9830 | .9834 | . 9838 | . 9842 | . 9846 | .9850 | . 9854 | . 9857 |
| 2.2 | . 9861 | . 9864 | . 9868 | . 9871 | . 9875 | . 9878 | . 9881 | . 9884 | . 9887 | . 9890 |
| 23 | . 9893 | . 9896 | . 9898 | . 9901 | . 9904 | . 9906 | . 9909 | . 9911 | . 9913 | . 9916 |
| 2.4 | . 9918 | . 9920 | . 9922 | . 9925 | . 9927 | . 9929 | . 9931 | . 9932 | . 9934 | . 9936 |
| 2.5 | . 9938 | . 9940 | . 9941 | . 9943 | . 9945 | . 9946 | . 9948 | . 9949 | . 9951 | . 9952 |
| 2.6 | . 9953 | . 9955 | . 9956 | . 9957 | . 9959 | . 9960 | . 9961 | . 9962 | . 9963 | . 9964 |
| 2.7 | . 9965 | . 9966 | . 9967 | . 9968 | . 9969 | . 9970 | . 9971 | . 9972 | . 9973 | . 9974 |
| 2.8 | . 9974 | . 9975 | . 9976 | . 9977 | . 9977 | . 9978 | . 9979 | . 9979 | . 9980 | 9981 |
| 2.9 | . 9981 | . 9982 | . 9982 | . 9983 | . 9984 | . 9984 | . 9985 | . 9985 | . 9986 | . 9986 |
| 3.0 | . 9987 | . 9987 | . 9987 | . 9988 | . 9988 | . 9989 | . 9989 | . 9989 | . 9990 | . 9990 |
| 3.1 | . 9990 | . 9991 | . 9991 | . 9991 | . 9992 | . 9992 | . 9992 | . 9992 | . 9993 | . 9993 |
| 3.2 | . 9993 | . 9993 | . 9994 | . 9994 | . 9994 | . 9994 | . 9994 | . 9995 | . 9995 | . 9995 |
| 3.3 | . 9995 | . 9995 | . 9995 | . 9996 | . 9996 | . 9996 | . 9996 | . 9996 | . 9996 | . 9997 |
| 3.4 | . 9997 | . 9997 | . 9997 | . 9997 | . 9997 | . 9997 | . 9997 | . 9997 | . 9997 | . 9998 |

## SARDAR PATEL COLLEGE OF ENGINEERING

(Government Aided Autonomous Institute) Munshi Nagar, Andheri (W) Mumbai - 400058

END SEMESTER EXAMINATION MARCH 2023
Program: F. Y. M. Tech Givil with sts. cref
Course Code: AU-PG 01
Course Name: Project Planning and Management

Duration: 3 Hours
Maximum Points: 100
Semester: I

## Notes:

1. Answer any five questions.
2. All questions carry 20 points.

| Q.No. | Questions | Points | CO | BLModule <br> No. |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
|  | 1.1 Why is it important to issue a Civil, Structural, <br> Architectural Design Basis for the project? List ten of <br> the most important design requirements that should be <br> contained in the Design Basis. |  |  |  |  |
|  | 1.2 List out at least ten steps, in sequence, for the <br> preparation of Civil Structural, Architectural Tender <br> specification. | 10 | 1 | 1 | 2 |
|  | 2.1 Explain the three fundamental components in a <br> computer model for structural analysis. Explain the <br> three stages in the process of computer analysis <br> highlighting Engineer's and computer's roles. | 10 | 2 | 2 | 2 |
| 2 | 2.2 List out at least ten steps, in sequence, in the <br> designing of a complex plant steel structure. | 10 | 2 | 3 | 3 |
|  | 3.1 Write a detailed note on reinforcement Bar <br> Bending Schedules including contents, cutting length <br> and users. | 10 | 2 | 2 | 4 |
| 3 | 3.2 Explain Building Information Modelling (BIM) <br> along with its use in different stages of a construction <br> project and its advantages. | 10 | 2 | 1 | 5 |
|  | 4.1 Explain the key concepts for Project Resource <br> Management and Project Risk Management. | 10 | 1 | 2 | 2 |
|  | 4.2 Explain any five top Emerging Trends which are <br> impacting the Construction Industry today. | 10 | 3 | 5 | 5 |

Page 1 of 2

END SEMESTER EXAMINATION MARCH 2023

| Q.No. | Questions | Points | CO | BL | Module <br> No. |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 5 | 5.1 Explain the change in approach towards Project <br> Management in the seventh edition of the PMBOK <br> Guide briefly defining project management principles <br> and performance domains, list any three of each. | 10 | 3 | 2 | 1 |
|  | 5.2 List ten guidelines to be considered while <br> developing a plot plan/layout for a process plant. | 10 | 2 | 3 | 2 |
|  | 6.1 Write a brief note on i)Project and ii)Project <br> Management | 10 | 2 | 1 | 2 |
|  | 6.2 Explain Quality and Grade. Explain Quality <br> Assurance and Quality Control; which is preferred <br> and why? |  |  |  |  |
|  | 7.1 When a project commences, what are the early <br> activities carried out by CSA discipline? On what <br> activities does the CSA engineer spend a major <br> portion of time? | 10 | 4 | 4 | 5 |
| 7 | 7.2 Write a note on Method statement in construction. <br> What are the objectives of Constructability Reviews? | 10 | 2 | 3 | 2 |

